

Analysing the critical orientation of seismic loading in 3D buildings: preliminary results for constant lateral forces

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Abstract

The angle of seismic excitation has been proven to be an important factor when analysing the 3D behaviour of buildings. However, modern earthquake-related standards only cover partially the effects of this factor and practicable results of relevant studies are still limited. The proposed paper focuses on the determination of the critical angle of incidence based on the building's structural characteristics. An analytical expression is developed to define the critical angle for the case of single storey buildings and a special category of multi storey buildings under constant lateral forces assuming linear elastic behaviour of the structures.

Subject Headings. Civil Engineering, Engineering Structure, Seismic Monitoring

Author Keywords. Angle of Incidence, Earthquake Loading, Three-Dimensional Analysis, RC Buildings

1. Introduction

In real earthquake events the excitation angle of the seismic action is generally unknown a priori. Thus, it is prudent to perform seismic analysis by adopting a three-dimensional (3D) structural model and the angle of seismic incidence (ASI) that induces the highest structural demand. Although the consideration of such an angle, i.e. the critical angle of seismic incidence (ASI_{crit}), is required by earthquake-related standards (e.g. Eurocode 8 (2004)), no methodology is currently included in standard provisions for its determination. Alternatively, in order to account for the most unfavourable scenario without performing parametric analyses for every ASI, standards prescribe the simultaneous application of the earthquake loading along two orthogonal directions that coincide with the building's structural axes. Furthermore, in case the seismic action is represented by timewise non-simultaneous peak values, the combination of the responses of each direction is subsequently performed using well-known directional combination rules (Newmark 1975; Rosenblueth and Contreras 1977). The efficiency of these provisions as well as studies for the determination of the ASI_{crit} have been extensively addressed by several researchers over the past decades.

In the domain of linear elastic analysis, the ASI_{crit} was studied for response spectrum analysis (RSA) and for translational components having arbitrary shapes (Smeby and Der Kiureghian 1985; Lopez and Torres 1997). Particularly, it was proven that the ASI_{crit} does not depend on the ratio of the two horizontal components when they have the same shape nor on the arbitrary vertical component. For the determination of the ASI_{crit} , prior structural analyses with the response spectra under consideration were required to be carried out. In the same study the authors also demonstrated that different demand parameters obtain their maximum values for different ASI_{crit} . Menun and Der Kiureghian (1998) introduced the complete quadratic combination rule CQC3 to determine the ASI_{crit} and the corresponding maximum response from results obtained from preliminary analyses. The authors proved that

the traditional practice of considering only one ASI and using the directional combination rules (e.g. the 100/30 percentage rule) leads to unconservative results. Several studies dealing with different structures and various demand parameters can be found in the literature that demonstrate the inadequacy of the previously mentioned standard provisions (e.g. see Fernandez-Davila et al. 2000; Lopez et al. 2001; Menun and Der Kiureghian 2000). Based on those conclusions and still in the domain of response spectrum analysis, Lopez et al. (2000) determined upper boundaries for the structural demand with respect to the square root of the sum of the squares of the response for several structural demand parameters implementing the CQC3 rule. In the context of time history analysis (THA), the ASI_{crit} and the corresponding maximum response were determined by Athanatopoulou (2005) for structures with linear elastic behaviour. The author also showed that maximum values of each demand parameter are obtained for different $ASIs_{crit}$. Such as for the case of RSA, the determination of the ASI_{crit} was only achieved after performing preliminary analyses of the structure using the ground motion records under consideration. Additionally, the inadequacy of considering only one orientation of the seismic action and using the directional combination rules was also shown by Kostinakis et al. (2013) using results of THA. Furthermore, studies dealing with the lateral force analysis (LFA), which is considered the simplest as well as the most conservative method of analysis to determine structural demand under earthquake loading, showed that the demand may be underestimated if the ASI_{crit} is not taken into account (Morfidis et al. 2008; Quadri and Madhurin 2014).

When nonlinear analysis is performed, the determination of the ASI_{crit} can no longer be achieved by performing preliminary analyses (Cantagallo et al. 2012; Fontara et al. 2015). Applications involving the nonlinear time history analysis (NLTHA) of several structures showed that the inelastic structural demand may be substantially underestimated when the ground motion components are applied along the building's global structural axes (MacRae and Mattheis 2000; Zaghlool et al. 2001; Hosseini and Salemi 2008; Fernandez-Davila and Cruz 2008; Magliulo et al. 2014). Moreover, it was shown that directional combination rules are highly dependent on the selection of the ASI and may considerably underestimate the structural response. In order to take into account the variability of the ASI in NLTHA, Lagaros (2010) implemented multicomponent incremental dynamic analysis and showed that considering thirty ASIs and record pairs is adequate enough to account for the randomness of both the record and the ASI. More recently, Sebastiani et al. (2014) introduced a simplified approach to evaluate the ASI_{crit} by performing parametric pushover analyses in single degree of freedom systems representing a multi degree of freedom structure in an attempt to reduce the computational effort.

Overall, past research has shown that for all methods of analysis and material behaviour the maximum response may not occur when the ground motion components are applied along the global structural axes. Furthermore, available results also showed that existing directional combination rules may underestimate the true structural response. Since no correlation has been found yet between the building's configuration and the expected response due to bidirectional seismic input, practitioners must perform various analyses in order to simulate several ASIs and establish the relevant critical orientation; with the latter being particularly demanding when nonlinear material behaviour is considered. Existing methods developed to explicitly determine the ASI_{crit} correspond to structures with linear elastic behaviour and still require the results of preliminary analyses. As a result, the process of seismic design or safety assessment of a structure may result in an overly time-consuming numerical procedure. It is

thus necessary to focus on the definition of new methods in order to account for the ASI_{crit} in a more robust and efficient way.

In light of this, a preliminary approach for the direct calculation of the ASI_{crit} is presented in the context of linear elastic analysis and for constant static lateral loading. The proposed procedure is defined in the context of the seismic safety assessment of existing buildings and establishes critical demand criteria for single storey buildings and for a special category of multi-storey buildings based on maximum storey displacements. The proposed approach defines an expression to determine the ASI_{crit} based only on the geometrical and material characteristics of the building, thus being independent of the external static loading.

2. Review of concepts regarding the static behaviour of single storey buildings

The developed methodology for the determination of the ASI_{crit} is based on the static behaviour of 3D single storey buildings, non-symmetric in plan, as described by Roussopoulos (1932). The behaviour of single storey buildings is of particular importance in the study of earthquake resistant structures, since it was used extensively in the past and it is still used nowadays in the framework of LFA as a proxy for the behaviour analysis of multi-storey structures. For this reason, some basic concepts of single storey structures and of their behaviour under static loading are presented first to help illustrating the rationale. The mechanical behaviour of the materials will be considered to be linear elastic and the floor will be considered to be in-plane rigid and out-of-plane flexible. Furthermore, the vertical elements will be considered to be axially rigid. Single storey buildings with the aforementioned properties always possess an elastic centre C_s and principal axes (I, II, III), where C_s is the intersection of the vertical principal axis III (also termed elastic axis) with the floor diaphragm (Roussopoulos 1932; Makarios and Anastassiadis 1998).

For a single storey building with the aforementioned properties, the total number of degrees of freedom (dof) representing global behaviour can be considered to be three: two translational and one rotational. As a result, the horizontal stiffness of the structure can be described by a 3×3 matrix with respect to an arbitrary reference system O (X,Y,Z), where X-Y is the horizontal plane, Z is the vertical axis and O is their intersection point. For an arbitrary horizontal loading vector $[F_x, F_y, M_z]^T$, the following static equilibrium has to be satisfied:

$$\begin{bmatrix} K_{xx} & K_{xy} & K_{xz} \\ K_{yx} & K_{yy} & K_{yz} \\ K_{zx} & K_{zy} & K_{zz} \end{bmatrix} \cdot \begin{bmatrix} u_x \\ u_y \\ \theta_z \end{bmatrix} = \begin{bmatrix} F_x \\ F_y \\ M_z \end{bmatrix} \quad (1)$$

where K_{ij} are the stiffness matrix coefficients and $[u_x \ u_y \ \theta_z]^T$ is the displacement vector with respect to the O (X,Y,Z) reference system.

Based on the properties of C_s for single storey buildings (Roussopoulos 1932; Makarios and Anastassiadis 1998), it is always possible to define a special reference system C_s (I, II, III) with respect to which the stiffness matrix takes a diagonal form. This reference system is the principal system of the structure and is defined by a horizontal plane I-II, the vertical axis III and their intersection point C_s . In the principal reference system, the static equilibrium takes the form defined by:

$$\begin{bmatrix} K_I & 0 & 0 \\ 0 & K_{II} & 0 \\ 0 & 0 & K_{III} \end{bmatrix} \cdot \begin{bmatrix} u_I \\ u_{II} \\ \theta_{C_s} \end{bmatrix} = \begin{bmatrix} F_I \\ F_{II} \\ M_{C_s} \end{bmatrix} \quad (2)$$

where K_I , K_{II} , K_{III} are the principal stiffness matrix coefficients and $[u_I \ u_{II} \ \theta_{C_s}]^T$ and $[F_I \ F_{II} \ M_{C_s}]^T$ are the displacement and the loading vectors, respectively, with respect to the C_s (I, II, III) principal reference system.

The principal stiffness matrix is derived from translation and rotation of the initial stiffness matrix defined by Eq. **Error! Reference source not found.** and the location of the principal system is determined from the conditions $K_{II,III} = 0$, $K_{I,III} = 0$ and $K_{I,II} = 0$. The coordinates of C_s with respect to the initial system are calculated by the first two conditions, while the third condition defines the rotation angle ω of the principal axis I with respect to the structural axis X (Anastassiadis 1989).

3. Determination of the critical angle of incidence in single storey buildings

3.1. Principal axes parallel to global axes

In the special case where all force-resisting elements have their axes along two perpendicular directions X and Y, the stiffness coefficients K_{XY} and K_{YX} of Eq. **Error! Reference source not found.** are equal to zero. Therefore, the angle ω is also equal to zero ($\omega = 0^\circ$) and the principal axes are derived from the initial global axes only by translation, i.e. without rotation.

Based on the properties of C_s (Roussopoulos 1932; Anastassiadis 1989), a unit static force passing through the centre of stiffness causes a translation without rotation of the floor diaphragm. During the rotation of a unit static force around C_s , the peak displacement vector defines an ellipse with axes I, II and lengths of the semi-axes equal to $f_I (= 1/K_I)$ and $f_{II} (= 1/K_{II})$, respectively. In addition, a unit torsional moment about a vertical axis causes rotation of the diaphragm about C_s . Therefore, a horizontal static force F rotating around any point of the diaphragm, e.g. point O in Figure 1, will cause an elliptic translation of C_s and a rotation of the diaphragm around C_s . Hence, the overall displacement of the floor may be obtained as a superposition of two states of pure translation within the planes (I-III, II-III) and of one state of pure rotation about the axis III, as illustrated in Figure 1. The value of the moment that causes rotation will depend on the orthogonal distance d between F and C_s (Figure 1).

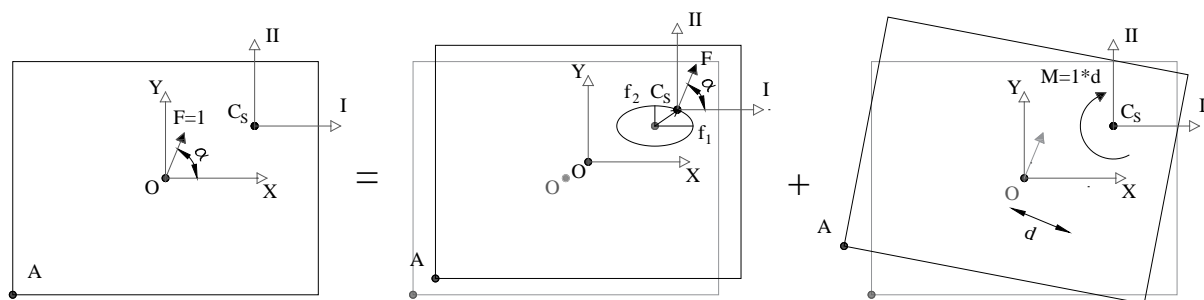


Figure 1: Displacement of a single storey building subjected to a horizontal unit static force applied at O with an arbitrary direction with respect to the structural axes X-Y. The principal axes I-II are parallel to the structural axes

According to the previously mentioned, the vector of displacements of C_s $[u_X^{C_s} \ u_Y^{C_s} \ \theta_Z]^T$ due to the rotation of a unit static force about O may be calculated using:

$$\begin{bmatrix} u_X^{C_s} \\ u_Y^{C_s} \\ \theta_Z \end{bmatrix} = \begin{bmatrix} \frac{\cos(\alpha)}{K_I} \\ \frac{\sin(\alpha)}{K_{II}} \\ \frac{\cos(\alpha) \cdot y_{C_s} - \sin(\alpha) \cdot x_{C_s}}{K_{III}} \end{bmatrix} \quad (3)$$

where y_{C_s} and x_{C_s} represent the coordinates of C_s with respect to the global reference system $O(X,Y,Z)$ and α is the angle of the horizontal static force with respect to the global X axis, which also coincides with the principal axis I . The displacement of a generic point of the diaphragm, e.g. point A in Figure 1, may then be calculated according to Eq. 4 which defines the displacement of A as a function of the displacement of C_s based on rigid body kinematics:

$$\begin{bmatrix} u_X^A \\ u_Y^A \\ \theta_Z \end{bmatrix} = \begin{bmatrix} u_Y^{C_s} - y_A \cdot \theta_Z \\ u_Y^{C_s} + x_A \cdot \theta_Z \\ \theta_Z \end{bmatrix} \quad (4)$$

where y_A and x_A are the coordinates of A with respect to the principal coordinate system $C_s(I, II, III)$ which, in the current case, is parallel to the global system ($\omega=0^\circ$). Subsequently, the total resultant horizontal displacement of A , u^A , can finally be derived by applying the Pythagorean Theorem:

$$u^A = \sqrt{(u_X^A)^2 + (u_Y^A)^2} = \sqrt{\left(\frac{\cos(\alpha)}{K_I} - y_A \frac{(\cos(\alpha) \cdot y_{C_s} - \sin(\alpha) \cdot x_{C_s})}{K_{III}} \right)^2 + \left(\frac{\sin(\alpha)}{K_{II}} + x_A \frac{(\cos(\alpha) \cdot y_{C_s} - \sin(\alpha) \cdot x_{C_s})}{K_{III}} \right)^2} \quad (5)$$

It can be seen that the displacement of A is defined based on the known geometrical and mechanical characteristics of the structure and it is a function of the unknown angle α . Consequently, the angle that leads to the maximum displacement can be calculated straightforwardly by differentiating Eq. 5 with respect to the angle α and by setting this derivative equal to zero:

$$\frac{du^A}{d\alpha} = 0 \rightarrow \alpha_{crit} \quad (6)$$

The calculated angle α_{crit} corresponds to the ASl_{crit} for the displacement of the generic point A . Since the maximum displacement of the whole diaphragm will occur at the point with the largest distance from C_s , the ASl_{crit} of that point will coincide with the ASl_{crit} of the building when considering the maximum displacement of the building as a measure of the global building performance.

3.2. Principal axes not parallel to global axes

Generally, structures may have structural elements that do not have their local axes along two perpendicular directions. Hence, a common reference system may be difficult to be defined. Also, in this case, the terms K_{XY} and K_{YX} of Eq. 1, which correspond to a global - arbitrarily defined - reference system, are no longer equal to zero. Therefore, the angle ω is no longer equal to zero and the principal axes I and II are rotated with respect to axes X and Y , as illustrated in Figure 2.

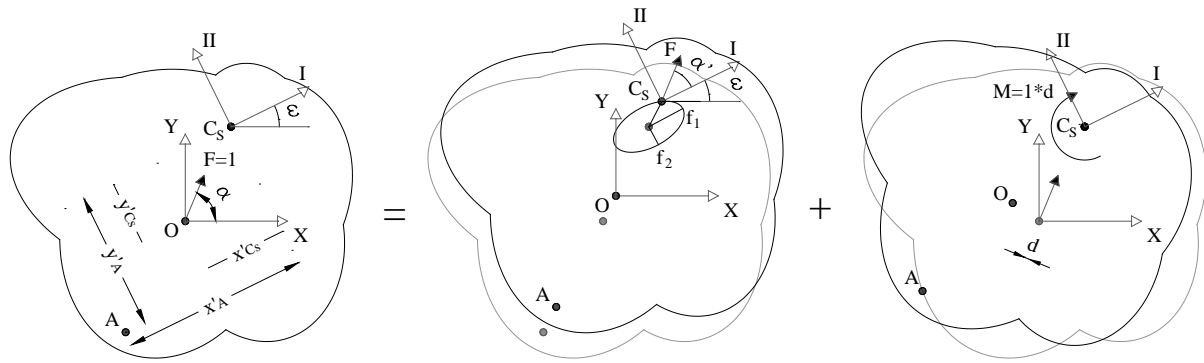


Figure 2: Displacement of a single storey building subjected to a horizontal unit static force applied at O with an arbitrary direction with respect to the structural axes X-Y. The principal axes I-II are not parallel to the structural axes

For the implementation of the methodology presented in Section 3.1 the determination of the coordinates of C_s , the angle ω and the principal stiffness matrix are again performed according to the procedure presented in Section 2. Subsequently, the coordinates of C_s with respect to the reference system O (X,Y,Z) are determined in the rotated system of axes (x'_{Cs} , y'_{Cs}), as well as the coordinates of point A (x'_A , y'_A) with respect to the principal coordinate system C_s (I, II, III). For structures with principal axes that are not parallel to the global axes, the displacement vector of C_s due to the rotation of a unit force about O is defined by:

$$\begin{bmatrix} u_x^{Cs} \\ u_y^{Cs} \\ \theta_z \end{bmatrix} = \begin{bmatrix} \frac{\cos(\alpha')}{K_I} \\ \frac{\sin(\alpha')}{K_{II}} \\ \frac{\cos(\alpha') \cdot y'_{Cs} - \sin(\alpha') \cdot x'_{Cs}}{K_{III}} \end{bmatrix} \quad (7)$$

where α' represents the angle of the static force with respect to the principal axis I shown in Figure 2. The total resultant displacement of a generic point A (shown in the same figure) can then be calculated using rigid body kinematics by:

$$u^A = \sqrt{(u_x^A)^2 + (u_y^A)^2} = \sqrt{\left(\frac{\cos(\alpha')}{K_I} \cdot y'_A - \frac{(\cos(\alpha') \cdot y'_{Cs} - \sin(\alpha') \cdot x'_{Cs})}{K_{III}} \right)^2 + \left(\frac{\sin(\alpha')}{K_{II}} + x'_A \frac{(\cos(\alpha') \cdot y'_{Cs} - \sin(\alpha') \cdot x'_{Cs})}{K_{III}} \right)^2} \quad (8)$$

Similarly to the procedure presented in Section 3.1, the ASl_{crit} of the static horizontal force that leads to the maximum response in terms of horizontal displacements of the diaphragm can be derived by differentiating Eq. **Error! Reference source not found.** with respect to the angle α' and by solving this derivative equal to zero. Since the angle α' is defined with respect to axis I of the principal coordinate system, it needs to be converted into the angle α defined in terms of the global coordinate system O (X,Y,Z) by adding the angle ω :

$$\alpha = \alpha' + \omega \quad (9)$$

4. Determination of the critical angle of incidence in multi-storey buildings

General multi-storey buildings do not have an elastic axis or principal bending directions (Riddel and Vasequez 1984; Anastassiadis 1989), therefore the methodology presented in Section 3 can't be fully implemented for this type of structures. Nevertheless, there are special categories of multi-storey buildings for which those concepts are still usable. For those cases,

the expressions connecting the ASl_{crit} with the geometrical and mechanical characteristics of the buildings are still applicable after performing some modifications. Buildings for which an elastic axis and principal bending planes can be defined belong to one of the following categories of systems (Riddel and Vasquez 1984; Makarios and Anastassiadis 1998; Athanatopoulou and Doudoumis 2008): systems with two horizontal axes of symmetry in plan view, isotropic systems, ortho-isotropic systems and complex-isotropic (coaxial) systems.

All the previously mentioned systems have principal bending planes (I-III, II-III) the intersection of which defines the elastic axis III of the system. Furthermore, the elastic axis of multi-storey buildings possesses elastic properties similar to those of the elastic axis of single storey buildings, summarized by Makarios and Anastassiadis (1998). Given these properties, the static response of the system is obtained by the superposition of two states of pure bending within the planes (I-III, II-III) and of one state of pure torsion about the axis III.

Generally, any N-storey building satisfies the following static equilibrium with respect to an arbitrary reference system (X,Y,Z) (Anastassiadis 1985):

$$\begin{bmatrix} K_{xx} & K_{xy} & K_{xz} \\ K_{yx} & K_{yy} & K_{yz} \\ K_{zx} & K_{zy} & K_{zz} \end{bmatrix} \cdot \begin{bmatrix} u_x \\ u_y \\ \theta_z \end{bmatrix} = \begin{bmatrix} f_x \\ f_y \\ m_z \end{bmatrix} \quad (10)$$

where u_x , u_y , θ_z are the N-dimensional vectors of translations u_{xi} , u_{yi} and rotations θ_{zi} with respect to the X_i , Y_i , Z_i axes, respectively, with $i = 1$ to N . Furthermore, f_x , f_y , m_z are the N-dimensional vectors of forces F_{xi} , F_{yi} and moments M_{zi} with respect to the X_i , Y_i , Z_i axes, respectively, with $i = 1$ to N . Finally, each K_{ij} element is a $N \times N$ stiffness matrix with i,j given by all the combinations of x , y and z .

In isotropic buildings, the horizontal stiffness matrices K_n of all elements have the following form (Anastassiadis 1985):

$$K_n = k_n \cdot K_0, \quad n=1,2,.. \quad (11)$$

where k_n is a numerical coefficient and K_0 is a constant reference matrix of order N (e.g. the stiffness matrix of an element of the system). Furthermore, it has been proven that matrices K_{ij} in Eq. 10 have the following form (Anastassiadis 1985):

$$K_{ij} = k_{ij} \cdot K_0, \quad i,j=x,y,z \quad (12)$$

where the coefficients $k_{ij} = k_{ji}$ are calculated as functions of k_n . The static equilibrium in Eq. 10 of the N-storey building with respect to the global reference system (X,Y,Z) may then be rewritten as presented in Eq. 13 after replacing Eqs. 11 and 12 in Eq. 10.

$$\begin{bmatrix} k_{xx}K_0 & k_{xy}K_0 & k_{xz}K_0 \\ k_{yx}K_0 & k_{yy}K_0 & k_{yz}K_0 \\ k_{zx}K_0 & k_{zy}K_0 & k_{zz}K_0 \end{bmatrix} \cdot \begin{bmatrix} u_x \\ u_y \\ \theta_z \end{bmatrix} = \begin{bmatrix} f_x \\ f_y \\ m_z \end{bmatrix} \quad (13)$$

It has been shown that this equilibrium may always take the following form (Anastassiadis 1985):

$$\begin{bmatrix} k_I K_0 & 0 & 0 \\ 0 & k_{II} K_0 & 0 \\ 0 & 0 & k_{III} K_0 \end{bmatrix} \cdot \begin{bmatrix} u_I \\ u_{II} \\ \theta_k \end{bmatrix} = \begin{bmatrix} f_I \\ f_{II} \\ m_k \end{bmatrix} \quad (14)$$

where k_I , k_{II} , k_{III} are calculated as defined as in Section 2 by replacing K_{ij} of Eq. 1 with k_{ij} of Eq. 13. The terms f_I , f_{II} and m_k are the N-dimensional vectors of the loads and u_I , u_{II} and θ_k are the

N-dimensional vectors of displacements with respect to the principal reference system (I, II, III). The coordinates of the location of the elastic axis and the orientation angle ω of the horizontal principal axes (I, II) are then determined from the conditions $k_{II,III} = 0$, $k_{I,III} = 0$ and $k_{I,II} = 0$, respectively. Based on these concepts, the procedure detailed in Section 3 can be seen to be applicable for the determination of the ASl_{crit} by replacing the terms of the principal stiffness matrix (K_I , K_{II} , K_{III}) of Eq. 2 by the k_I , k_{II} , k_{III} coefficients of Eq. 14. Finally, it is noted that for ortho-isotropic and complex-isotropic systems, a similar procedure to the one presented herein to define the global stiffness matrix of Eq. 13 can be established as described in the study of Anastassiadis (1985).

5. Case study applications

Two numerical examples will be presented hereafter to illustrate the applicability of the proposed methodology in single storey and multi storey buildings with isotropic properties. In both example structures the modulus of elasticity is considered to be equal to 25 GPa.

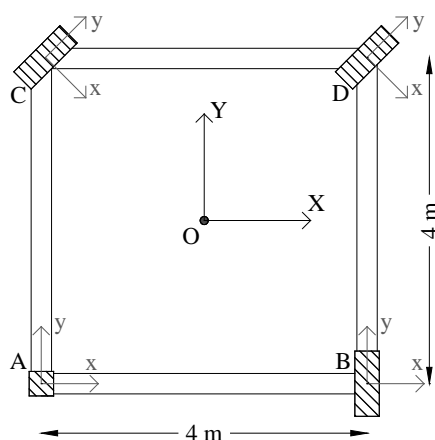
5.1. Single storey building

The first case study comprises a non-symmetric single storey structure. The plan view as well as the material and the geometrical characteristics of the structure are presented in Figure 3. Assumptions regarding the mechanical behaviour of the materials described in Section 2 are valid as well. The local structural axes of columns C and D are rotated -45° about the vertical axis Z and all columns are considered fixed to the ground. This structure doesn't correspond to a realistic building, but was chosen in order to represent a complex case study where the principal directions do not coincide with the main structural axes.

Following the procedure described in Section 3.2 the lateral stiffness matrix of the structure is determined in the global coordinate system O (X,Y,Z). Subsequently, the coordinates of the elastic centre C_S , $x_{C_S} = 0.6209$ (m) and $y_{C_S} = 1.3922$ (m), and the angle $\omega = -31.58^\circ$ that defines the direction of principal axis I are determined in the same coordinate system. Next, the principal stiffness matrix is computed along with the coordinates of C_S , x'_K and y'_K , with respect to O and the rotated axes (

Figure 4a), as well as the coordinates of A, x'_A and y'_A , with respect to the principal reference system C_S (I, II, III) (

Figure 4b).



Height	4.00 m
E (modulus of elasticity)	25 GPa
Column A	0.30 x 0.30 (b x h) (m ²)
Column B	0.30 x 0.80 (b x h) (m ²)
Column C	0.30 x 0.80 (b x h) (m ²)
Column D	0.30 x 0.80 (b x h) (m ²)
Beams	0.25 x 0.60 (b x h) (m ²)

Figure 3: Plan view and material and geometric characteristics of the single storey structure

By introducing the coordinates and the principal stiffness matrix coefficients into Eq. 8 the resultant displacement of the column corresponding to point A can be calculated for every value of α' . The derivative of the equation, solved with the mathematical software Maxima (Maxima 2014), leads to the ASI_{crit} with respect to the principal reference system $\alpha'_{crit} = 182.3^\circ$. The ASI_{crit} with respect to the X-Y reference system is determined from Eq. 9 equal to 150.7° .

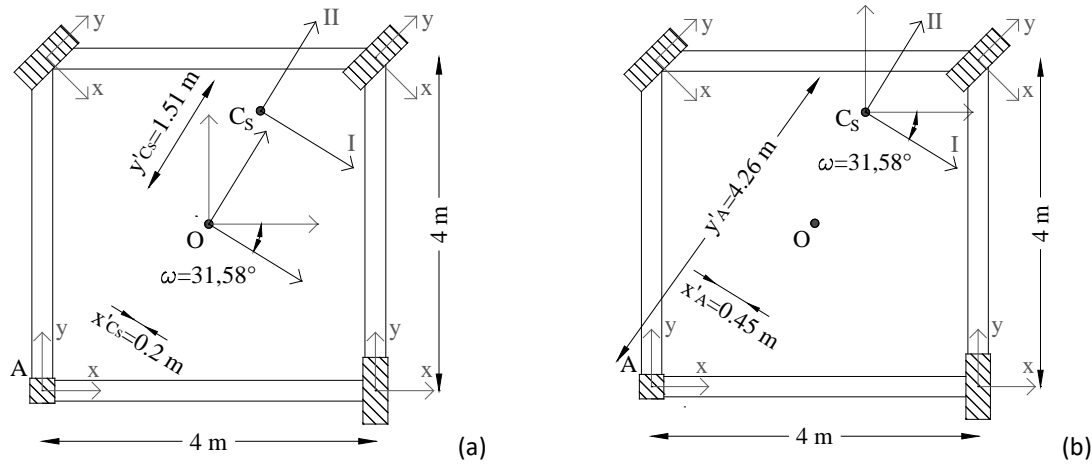


Figure 4: Determination of the CS (a) and A (b) coordinates

In order to verify the results obtained analytically, the structure is also simulated using the software OpenSees (McKenna et al. 2000) and a parametric analysis is performed by varying the ASI of a horizontal static load applied at the geometrical centre of the floor, O. The value of the force is assumed to be equal to 1kN and the ASI varies between 0° and 360° in steps of 0.5° . The variation of the displacement of point A with the ASI is shown in Figure 5. The maximum displacement is achieved for an ASI equal to 150.5° , thus validating the analytical result. It is noted that a further discretization of the angle step for the parametric analysis would lead to the exact same angle.

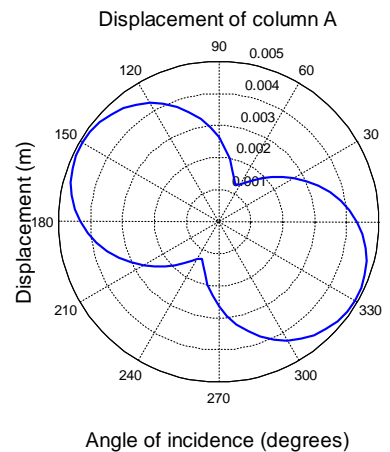


Figure 5: The displacement of column A due to the rotation of a unit static force applied at O about the vertical axis Z

5.2. Multi-storey isotropic building

The second case study comprises a 3-storey isotropic building complying with the mechanical and material characteristics defined in Section 2. The plan view of a typical storey of the considered structure is presented in Figure 6a. The building consists of identical planar frames, shown in Figure 6b, which are connected at floor levels by rigid horizontal diaphragms. The height of all storeys is constant and equal to 3.50 m, the cross section dimensions of the columns are $0.30 \times 0.30 \text{ m}^2$ and those of the beams are $0.25 \times 0.60 \text{ m}^2$.

Since the vertical resisting elements have proportional stiffness matrices, the building is an isotropic building and has a real elastic axis and principal bending directions. The lateral stiffness matrix with respect to the global reference system (X,Y,Z) is given by:

$$\underline{K} = \begin{bmatrix} 3 \cdot \underline{K}_0 & 0 \cdot \underline{K}_0 & -6 \cdot \underline{K}_0 \\ 0 \cdot \underline{K}_0 & 4 \cdot \underline{K}_0 & 5 \cdot \underline{K}_0 \\ -6 \cdot \underline{K}_0 & 5 \cdot \underline{K}_0 & 249 \cdot \underline{K}_0 \end{bmatrix} \quad (15)$$

where \underline{K}_0 is the following 3x3 lateral stiffness matrix of an individual frame:

$$\underline{K}_0 = \begin{bmatrix} 55664.8 & -32738.2 & 1769.6 \\ -32738.2 & 61642.2 & -30359.0 \\ 1769.6 & -30359.0 & 288668.1 \end{bmatrix} \quad (16)$$

The coordinates of the elastic axis, $x_{Cs} = 1.25$ m and $y_{Cs} = 2.00$ m, are calculated in the global coordinate system (Figure 6a). The angle ω between the principal direction I and the global reference axis X is equal to zero since all resisting elements have their local structural axes parallel to the X or Y structural axes. The lateral stiffness matrix with respect to the principal bending planes defined by Eq. 14 is then:

$$\underline{K}_{I,II,III} = \begin{bmatrix} 3 \cdot \underline{K}_0 & 0 & 0 \\ 0 & 4 \cdot \underline{K}_0 & 0 \\ 0 & 0 & 230.75 \cdot \underline{K}_0 \end{bmatrix} \quad (17)$$

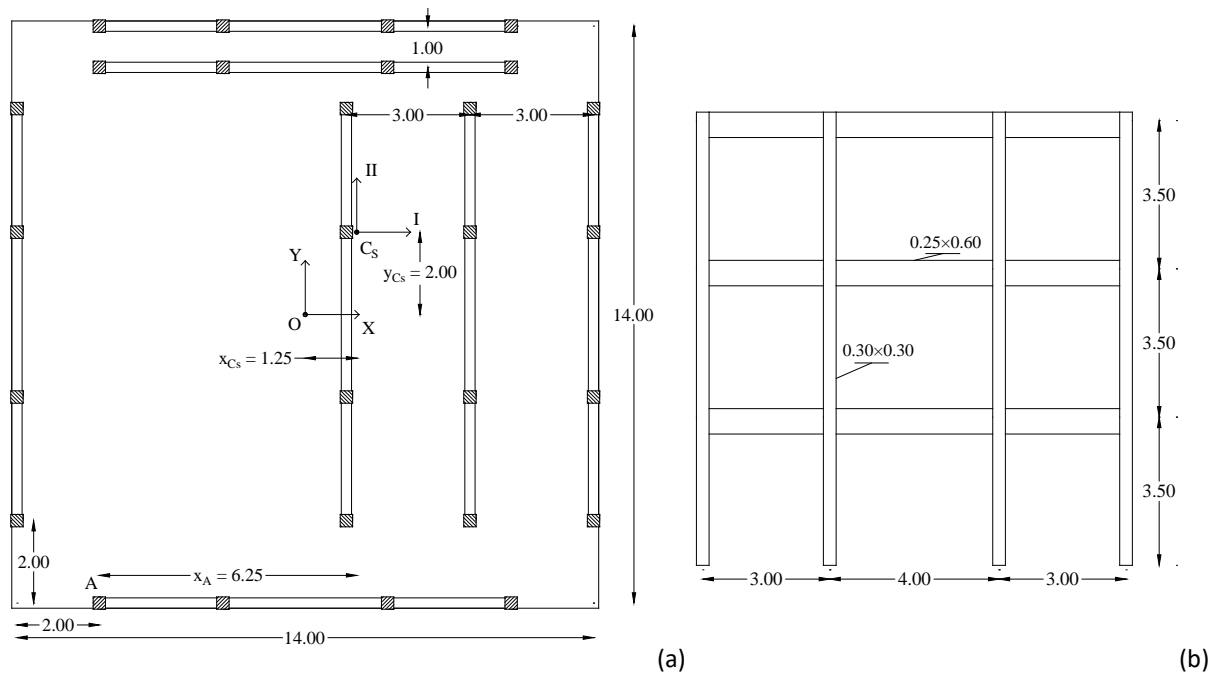


Figure 6: Plan view of a typical floor of the 3-storey isotropic building (a) and typical frame (b)
(dimensions are in m)

Subsequently, the coordinates of point A, x_A and y_A , with respect to C_s in the principal coordinate system are calculated (see Figure 6a). Point A corresponds to the top of the column that has the maximum distance from the elastic axis in each floor, thus leading to the maximum displacement in each diaphragm.

By implementing Eq. 5 for systems having all their force resisting element axes parallel to the global axes, differentiating it with respect to α and solving the derivative equal to zero, the ASI_{crit} leading to the maximum displacement is determined. The derivative and the equation

is solved with the Maxima software (Maxima 2014). The ASI_{crit} with respect to the principal axes (I, II, III) is then found to be equal to 160.75° .

In order to verify the results obtained analytically, the structure is also simulated using the software OpenSees (McKenna et al. 2000) and a parametric analysis is performed by varying the ASI of a lateral distribution of horizontal static loads applied at the geometrical centres of the floor diaphragms. The value of the force at each diaphragm is assumed to be equal to 1 kN and the ASI varied between 0° and 360° in steps of 5° . It is noted that the lateral distribution pattern of the horizontal forces only affects the magnitude of the displacements, not the ASI_{crit} , since the analysis is linear elastic and the building under consideration has a real elastic axis. The maximum displacements of column A for each floor and the corresponding ASI_{crit} , obtained using the Pythagorean Theorem, are determined and are presented in Figure 7.

According to the parametric analysis, although the values of the maximum displacements for every storey are, as expected, different, the ASI_{crit} is found to be 160.0° for all storeys, thus validating the value obtained from the analytical expression. The maximum displacement of the different diaphragms occurs under the same ASI_{crit} because of the isotropic properties of the building. As noted for the previous example, a further discretization of the angle step for the parametric analysis would lead to the same angle obtained from the analytical expression.

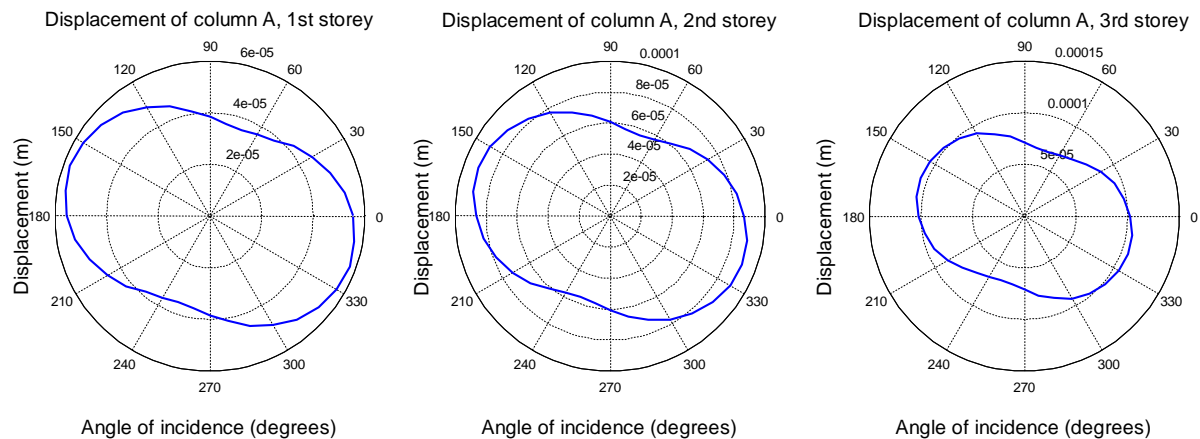


Figure 7: The displacement of column A for each storey due to the rotation of a lateral distribution of unit static force applied at the centres of mass of each diaphragm about the vertical axis Z

6. Concluding remarks

In the current paper an analytical framework was developed to determine the ASI_{crit} in single storey buildings and multi-storey buildings with isotropic properties analysed with linear static analysis. Given that past research has shown that each demand parameter obtains its maximum value for a different ASI, the proposed approach only examined the storey displacements of the structure, including the top displacement, a demand parameter that was found suitable to describe global 3D structural performance. The validity of the developed framework was demonstrated by two case studies, a single storey building and a 3-storey isotropic building. The applicability of the proposed framework is straightforward since it depends only on the materials and the geometrical characteristics of the structure and it is independent of the external static force. An additional advantage of the methodology includes its validity for forces applied at any point of the diaphragm, thus allowing for the consideration of accidental eccentricities.

Although the presented framework introduces a direct methodology to determine the ASI_{crit} , its applicability is still bounded by the requirement for constant lateral forces and the

existence of a real elastic axis. During LFA, the horizontal seismic forces may change in each direction depending on the response spectrum and the fundamental periods of vibration of the structure along the considered direction. As a result, the proposed methodology leads to accurate results in structures having their principal translational periods (periods that correspond to the principal directions) on the constant acceleration branch of the spectrum, in accordance with Morfidis et al. (2008). An extension of the methodology is currently being developed by the authors in which the shape of the response spectrum is introduced into the analytical expressions.

Regarding the existence of an elastic axis, it has been proven a prerequisite for the uncoupling of the dofs and thus the application of the methodology. However, in general multi storey buildings the nullification of the diaphragm rotations for translational excitation with horizontal static forces applied along a vertical axis is not possible. Since the elastic axis is a very good descriptor of a building and allows the determination of its static behaviour independently of the external loading (Makarios and Anastassiadis 1998), studies have been dedicated to extend and generalize its concept for general multi-storey buildings. Two different methodologies can be found in the literature that deal with the approximation of the elastic axis for multi-storey buildings (Makarios and Anastassiadis 1998; Marino and Rossi 2004; Doudoumis and Athanatopoulou 2008). The advancement of the proposed methodology with exploitation of these concepts constitutes an area of future research.

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